# DEFORMATION MAPS FOR BOLTED T-STUBS

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# ABSTRACT

Deformation maps on the plastic and ultimate failure of T-stubs with a single bolt-row in tension are developed. The maps condense a large body of information within a two dimensional parameters space onto which different modes of deformation, including the régime boundaries, are plotted for any practical combination of geometric and material properties encountered in a T-stub. Its fidelity is demonstrated with experimental data from literature, and through a detailed parametric investigation by high-fidelity finite element analysis. The predictive capability of two existing analytical models are also assessed against predictions by the finite element model. Their shortcomings, and applicability, are critically assessed and discussed.

Keywords: T-stubs, Eurocode 3, Deformation maps, Failure modes, Joints.

# **INTRODUCTION**

A bolted end plate connection is a class of moment resisting connections that is widely encountered in steel-framed structures. Its overall resistance is offered through a combination of tensile forces that act in the bolts adjacent to one flange,

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and compressive forces experienced by the bearing at the other. Unless significant 5 catenary actions develop in its adjoining beam, these tensile and compressive forces 6 are typically assumed to be equal and opposite. In general, the rotational capacity 7 of any bolted joint is limited by the deformation within its tension zone which 8 comprises of the column flange, end-plate and bolts in tension. In Eurocode 3 9 (EN 1993-1-8 2005) - denoted hereinafter as EC3 - the tension zone is modelled 10 by a T-stub and the ability to predict, a priori, its mode of deformation for a 11 broad range of material (T-stub and bolt) and geometric combinations is integral 12 to characterising the deformation capacity of a structural joint. 13

For T-stubs comprising of a single bolt-row in tension, three modes of failure, 14 or régimes of deformation, can develop: mode 1 - complete yielding of the flange; 15 mode 2 - bolt failure with yielding of the flange; and, mode 3 - bolt failure. Within 16 the constitutive framework of limit analysis, Piluso et al. (2001) showed that each 17 mode corresponds to a unique range of non-dimensional parameter  $\beta$  (Eq. 1). To 18 extend the aforesaid to include material strain-hardening and 'ultimate' failure 19 prediction, Piluso et al. (2001) adopted a piece-wise linear approximation of the 20 true stress-strain curve of the flange material. A disadvantage is that  $\beta$  must be re-21 evaluated – including the critical values corresponding to the transition between 22 régimes 1  $\rightarrow$  2  $(\beta_{\rm cr}^{1\rightarrow 2})$  and 2  $\rightarrow$  3  $(\beta_{\rm cr}^{2\rightarrow 3})$  – each time a different combination 23 of material and geometry (dimensions) is encountered. At present, there is no 24 straightforward means to visualise, and represent, the different modes and their 25 régime boundaries for a practical range of material and geometric parameters in a 26 compact design space that would be useful to designers. 27

Failure (both plastic and ultimate) prediction using  $\beta$  is a resistance-based approach that does not shed light on the deformation of the T-stub. To this end,

analytical models were developed by Piluso et al. (2001) and Francavilla et al. 30 (2016) – it must be emphasized that both neglect three-dimensional (3D) effects, 31 geometric non-linearity, moment-shear interaction, to name a few – where their 32 predictions were shown to agree well with the force-displacement curves of T-stubs 33 failing in mode 1. However, the model by Piluso et al. (2001) overestimates the 34 ultimate displacement of T-stubs that fail in mode 2, largely as a consequence of 35 neglecting displacement compatibility between the bolt and flange. This simplifi-36 cation was relaxed by Francavilla et al. (2016) which reduces the overestimation 37 of  $\Delta$  in mode 2. No experimental data were available in mode 3; consequently, 38 comparison to analytical predictions was not performed. 39

In this paper, we exploit the dimensionless parameter  $\beta$  (Piluso et al. 2001) to 40 construct deformation maps on the plastic and ultimate failure of T-stubs with a 41 single bolt-row in tension. The fidelity of the deformation maps will be demon-42 strated through experimental data from Girão Coelho (2004), Bursi and Jaspart 43 (1997) and Piluso et al. (2001). Further validation is provided through a detailed 44 parametric investigation by three-dimensional finite element (FE) modelling, which 45 considers material damage in both the flange and bolt, as well as geometric non-46 linearity. Predictions by the FE model are used to assess the accuracy of the two 47 aforementioned analytical models. 48

# 49 DEFORMATION MAPS

The mode of deformation that develops in a T-stub is related to a non-dimensional parameter given by (Piluso et al. 2001)

$$\beta = \frac{2M}{mB} \tag{1}$$

where m is the distance from the axis of the bolt-hole to the plastic hinge at the

flange-to-web connection; M is the bending moment at the plastic hinges; and, B is the tensile force in each bolt shown in Fig 1. Since  $M = M(l_{eff}, t_p, f_y)$  and  $B = B(A_s, f_{ub})$ , the parametric dependence for  $\beta$  can be expressed – by making use of dimensional analysis – as follows:

$$\beta = f\left(\frac{l_{eff}}{m}, \frac{f_y}{f_{ub}}, \frac{t_p^2}{A_s}\right) , \qquad (2)$$

which can be re-arranged to give

$$\frac{t_p^2}{A_s} = g\left(\beta, \frac{l_{eff}}{m}, \frac{f_y}{f_{ub}}\right) \tag{3}$$

where  $A_s$  is the tensile stress area of the bolt;  $f_y$  and  $f_{ub}$  are the yield strength 50 of the flange material and the ultimate strength of the bolt, respectively; and, 51  $t_p$  is the flange thickness. The effective length  $l_{eff}$  is the notional width defined 52 such that, at plastic collapse, the resistance of the flange – modelled as beams 53 - is equivalent to a T-stub whose kinematic mechanism is determined by the 54 yield line pattern that develops. Figure 2 shows the different possible yield line 55 patterns according to EN 1993-1-8 (2005). A circular pattern arises due to localised 56 action of the bolts; hence, they only develop in mode 1 where prying forces  $Q_1$ 57 are small compared to that  $(Q_2)$  in mode 2. By contrast, non-circular patterns 58 can develop in both modes 1 and 2, and are characterised by significant prying 59 forces that can cause premature bolt failure. In EN 1993-1-8 (2005), prying forces 60 are assumed to act along the edges of a T-stub at a distance n from the bolt 61 axis. According to McGuire and Winter (1978), a non-circular pattern will develop 62 whenever the ratio  $\lambda = n/m < 1.25$ . It is worth emphasising that both circular and 63 non-circular patterns are three-dimensional (3D) yielding mechanisms since their 64 hinge-line profile changes along the width of the T-stub, see Fig 2. By contrast, a 65 beam yield-line pattern is not and it develops whenever the width L is small, i.e. 66

L < 4m+1.25n. Here, only non-circular and beam patterns that induce significant prying forces are considered. Note that  $l_{eff}/m$  in Eq. 3 depends on the yield-line pattern that develops in the flange. For a non-circular pattern,  $l_{eff}/m = 4 + 1.25\lambda$ (EN 1993-1-8 2005). In a beam pattern, it is assumed, according to standard specifications for HE beams (UNI 5397:1978 1978), that  $L = 2.5 \cdot m$ .

To construct the maps, consider the plastic and ultimate failure of a T-stub separately. For a fully-plastic limit state, the corresponding  $\beta$  is given by (Piluso et al. 2001)

$$\beta = \beta_{Rd} = \frac{2 M_{pl,Rd}}{m B_{Rd}} , \qquad (4)$$

where  $B_{Rd}$  is the design tensile resistance of the bolt given by (EN 1993-1-8 2005)

$$B_{Rd} = 0.9 A_s f_{ub} \tag{5}$$

and  $M_{pl,Rd}$  is the design flexural resistance of the flange given by (EN 1993-1-8 2005)

$$M_{pl,Rd} = 0.25 \, l_{eff} \, t_p^2 \, f_y \; . \tag{6}$$

Substituting for  $B_{Rd}$  and  $M_{pl,Rd}$  in Eq. 4, and re-arranging according to the dimensionless groups in Eq. 3, one obtains:

$$\frac{t_p^2}{A_s} = \frac{1.8}{4 + 1.25\lambda} \ \beta_{Rd} \left(\frac{f_y}{f_{ub}}\right)^{-1} \qquad \text{Non-circular pattern}$$
(7a)

$$\frac{t_p^2}{A_s} = 0.72 \ \beta_{Rd} \left(\frac{f_y}{f_{ub}}\right)^{-1} \qquad \text{Beam pattern} \tag{7b}$$

Note that the relevant expression for  $l_{eff}/m$  corresponding to its respective yieldline pattern, given earlier, was substituted into Eq. 7. For an ultimate limit state, (Piluso et al. 2001)

$$\beta = \beta_u = \frac{2M_f}{mB_u} \,. \tag{8}$$

Here,  $B_u$  is the ultimate tensile resistance of the bolt given by (Piluso et al. 2001)

$$B_u = A_s f_{ub} \tag{9}$$

<sup>72</sup> and  $M_f$  is the bending moment at material fracture of the flange. Adopting an <sup>73</sup> idealised true stress-strain relation of the flange material in Fig. 3 – note that <sup>74</sup>  $\sigma_y(=f_y)$  and  $\sigma_u(=f_u)$  is the yield and ultimate tensile strength, respectively – <sup>75</sup> the bending moment at fracture can be expressed as (Piluso et al. 2001)

$$\frac{M_f}{M_y} = \kappa = \frac{1}{2} \left[ 3 - \left(\frac{\varepsilon_y}{\varepsilon_f}\right)^2 \right] + \frac{1}{2} \frac{E_h}{E} \left(\frac{\varepsilon_f - \varepsilon_h}{\varepsilon_y}\right) \left(1 - \frac{\varepsilon_h}{\varepsilon_f}\right) \left(2 + \frac{\varepsilon_h}{\varepsilon_f}\right) 
- \frac{1}{2} \frac{E_h - E_f}{E} \frac{\varepsilon_f - \varepsilon_u}{\varepsilon_y} \left(1 - \frac{\varepsilon_u}{\varepsilon_f}\right) \left(2 + \frac{\varepsilon_u}{\varepsilon_f}\right),$$
(10)

where the corresponding moment at yield is

$$M_y = \frac{l_{eff} t_p^2}{6} f_y \ . \tag{11}$$

The constant  $\kappa$  in Eq. 10 is a function of the flange material so that  $\kappa = 1$  at fully plastic condition. Following the same procedure as before, one obtains

$$\frac{t_p^2}{A_s} = \frac{3}{4+1.25\lambda} \ \beta_u \frac{1}{\kappa} \left(\frac{f_y}{f_{ub}}\right)^{-1}$$
Non-circular pattern (12a)

$$\frac{t_p^2}{A_s} = 1.2 \ \beta_u \frac{1}{\kappa} \left(\frac{f_y}{f_{ub}}\right)^{-1} \qquad \text{Beam pattern} \qquad (12b)$$

where the ratio  $\lambda \triangleq n/m$ . Régime boundaries are obtained by substituting  $\beta_{Rd}$ and  $\beta_u$  in Eqs. 7 and 12 with the corresponding expression for  $\beta$  in Table 1. The final expression for the régime boundaries has the following general form:

$$\frac{t_p^2}{A_s} = h(\lambda) \ \frac{1}{\kappa} \left(\frac{f_y}{f_{ub}}\right)^{-1} \tag{13}$$

<sup>79</sup> where  $h(\lambda)$  is tabulated in Table 2. Deformation maps for plastic and ultimate <sup>80</sup> failure of a T-stub – generated by plotting the régime boundaries (Eq. 13) in a

plot of  $t_p^2/A_s$  versus  $\kappa f_y/f_{ub}$  – are shown in Figs. 4 and 5, respectively. Notice 81 that each boundary is an isoline corresponding to a constant  $\lambda$  value. Any pair 82 of geometric  $(t_p^2/A_s)$  and material  $(\kappa f_y/f_{ub})$  parameters now uniquely locates a 83 point on the 2D map. From the map, one is able to determine the deformation 84 mode for a given value of  $\lambda$  (or by interpolation between any two  $\lambda$  values plotted 85 in Figs. 4 and 5, if required). Alternatively, the map allows a designer to select 86 the combination of geometric and material parameters for a T-stub to deform in 87 a desired mode. Note that only a single boundary demarcates the transition from 88 mode  $2\rightarrow 3$  for the beam pattern since it is independent of  $\lambda$ . The fidelity of the 89 maps will be validated later against experimental data from existing literature and 90 numerical predictions by FE model to be developed next. It is worth emphasising 91 that the deformation maps are only as accurate as the calculations/assumptions 92 undertaken in their generation. 93

## 94 FINITE ELEMENT MODELLING

Three-dimensional finite element (FE) models are developed for T-stubs sub-95 jected to quasi-static tension with ABAQUS/Standard V6.13. Predictions will be 96 validated against experimental data from three separate independent sources: viz. 97 WT1 (Girão Coelho 2004), T1 (Bursi and Jaspart 1997) and T15 (Ribeiro et al. 98 2015). The acronyms correspond to that used in their respective original source. 99 The aforesaid were performed for identical flange (S355 steel) and bolt (Grade 8.8) 100 materials, but each have a different  $\lambda$  (WT1: 0.9, T1: 1.0 and T15: 0.7). All three 101 failed in either mode 1 or 2 depending on the non-dimensional parameter  $t_p^2/A_s$ 102 which is different for each data-set. It is worth highlighting that both WT1 and 103 T1 contained two bolt-rows. Since they both develop a beam yield line pattern, 104

<sup>105</sup> 3D effects can be neglected and interactions between the two bolt-rows need not
<sup>106</sup> be considered (EN 1993-1-8 2005); consequently, they can be treated as equivalent
<sup>107</sup> T-stubs with a single bolt row.

## 108 Constitutive model

All the flange and bolts in the experiments were constructed from S355 steel 109 and Grade 8.8 bolt (of Young's modulus E = 210 GPa and Poisson's ratio  $\nu =$ 110 0.33), respectively. Their subsequent plastic responses are modelled using the 111 conventional  $J_2$  plasticity flow theory to allow implementation of the progressive 112 degradation of material stiffness. Figure 6 shows the nominal, and corresponding 113 true stress-strain curves of the S355 flange material. Characteristic points on the 114 nominal and true stress-strain curves are indicated as follows: yield (y), necking 115 (n), rupture (r - the last point on the softening branch just before the stress drops116 to 0) and fracture (f). An index *i* denotes a generic data point on the stress-strain 117 curve (nominal or true) connecting points y, n, r and f. The *i*-th data point 118 of the plastic response is extracted from the nominal stress-strain curve through 119 (ABAQUS 2009) 120

$$\tau_i = \begin{cases} \sigma_i^{\text{nom}}(1 + \varepsilon_i^{\text{nom}}), & y \le i < n \\ \sigma_n^{\text{nom}}(1 + \varepsilon_i^{\text{nom}}), & n \le i \le f \end{cases}$$
(14)

121 and

$$\varepsilon_i^{\rm pl} = \ln(1 + \varepsilon_i^{\rm nom}) - \varepsilon_y, \tag{15}$$

where  $\sigma_n^{\text{nom}}$  is the nominal stress at necking and  $\varepsilon_y$  is the strain at yield. To account for the effects of post necking strain localisation, we follow Pavlović et al. (2013)

 $\sigma$ 

124 by defining the nominal strain  $\varepsilon_i^{\text{nom}}$  as

$$\varepsilon_i^{\text{nom}} = \begin{cases} \Delta l_i / l_i, & y \le i < n\\ \varepsilon_{i-1}^{\text{nom}} + (\Delta l_i - \Delta l_{i-1}) / l_i, & n \le i \le f, \end{cases}$$
(16)

where  $l_i$  – given in Eq. 17 – represents the gauge length at the *i*-th data point and  $\Delta l_i = \varepsilon_i^{\text{nom}} l_0$ . For a cylindrical tensile coupon of diameter *d*, the initial gauge length  $l_0$  (= 50 mm) starts decreasing as the material softens. At the point of rupture, the gauge length becomes  $l_{\text{loc}} = 0.5d$  (Panontin and Sheppard 1999): here,  $l_{loc} = 6$  mm. Following Pavlović et al. (2013), the reduction of the gauge length is assumed to obey a power law through the localisation rate factor ( $\alpha_L = 0.5$ ) given by

$$l_{i} = \begin{cases} l_{0}, & y \leq i < n \\ \\ l_{0} + (l_{loc} - l_{0})[(\Delta l_{i} - \Delta l_{n})/(\Delta l_{r} - \Delta l_{n})]^{\alpha_{L}} & n \leq i \leq f \end{cases},$$
(17)

where  $\Delta l_n = \varepsilon_n^{\text{nom}} l_0$  and  $\Delta l_r = \varepsilon_r^{\text{nom}} l_0$  are the elongations of the gauge length at necking and rupture, respectively. The effects of strain localisation were not taken into account for the bolt since experiments by Girão Coelho (2004) have shown that the area reduction following the onset of necking affects the entire length of its shaft. Instead, the true stress-plastic strain curve obtained by Girão Coelho (2004) – this is plotted in Fig. 6 – is used for the bolt.

#### 138 Damage modelling

The damage initiation criterion by ABAQUS (2009) is used to predict the onset
of damage over a wide range of stress states, given by

$$\omega_d = \int \frac{d\bar{\varepsilon}^{\rm pl}}{\bar{\varepsilon}_0^{\rm pl}(\theta)} = 1 \tag{18}$$

where  $\omega_d$  is a state variable that increases monotonically with  $\bar{\varepsilon}^{\text{pl}}$  (expressed as a function of stress triaxiality  $\theta$ ). Here, the equivalent plastic strain at the onset of damage  $\bar{\varepsilon}_0^{\text{pl}}$  is defined as (Pavlović et al. 2013)

$$\bar{\varepsilon}_0^{\rm pl}(\theta) = \varepsilon_n^{\rm pl} \cdot \exp[-1.5(\theta - 1/3)] \tag{19}$$

where  $\varepsilon_n^{\text{pl}}$  is the true plastic strain at necking. In the post necking regime  $(n \leq i \leq f)$ , the damage process is controlled by the evolution of the damage variable  $D_i$  which is expressed as a function of the equivalent plastic displacement  $\bar{u}_i^{\text{pl}}$ . Following Pavlović et al. (2013),  $D_i$  is given by

$$D_i = \begin{cases} (1 - \bar{\sigma}_i / \sigma_i) \cdot 1.5, & n \le i \le r \\ 1, & i = f \end{cases}$$
(20)

where  $\sigma_i$  is the true stress (Eq. 14) and  $\bar{\sigma}_i = \sigma_i^{\text{nom}}(1 + \varepsilon_i^{\text{nom}})$  for  $y \leq i \leq f$ . Notice that no data points were considered between points r (rupture) and f (fracture) in Fig. 6 since rupture is defined as the last point on the softening branch of the nominal stress-strain curve, i.e.  $D_i = 1$  beyond point r.  $\bar{u}_i^{\text{pl}}$  is given by (Pavlović et al. 2013)

$$\bar{u}_i^{\rm pl} = \bar{u}_f^{\rm pl} (\varepsilon_i^{\rm pl} - \varepsilon_n^{\rm pl}) / (\varepsilon_f^{\rm pl} - \varepsilon_n^{\rm pl}) , \ n \le i \le f$$
(21)

<sup>153</sup> where the equivalent plastic displacement at fracture  $\bar{u}_f^{\rm pl}$  (in Eq. 21) is

$$\bar{u}_f^{\rm pl} = \lambda_{\rm S} \lambda_{\rm E} L_{\rm E} (\varepsilon_f^{\rm pl} - \varepsilon_n^{\rm pl}) \tag{22}$$

and  $L_{\rm E} = \sqrt[3]{V}$  (V is the volume of the element) is the characteristic length of the element (ABAQUS 2009, Sui et al. 2017). The factor  $\lambda_E$  must be calibrated to the element type used in the FE model. The dependence of  $\bar{u}_f^{\rm pl}$  on the mesh size is removed by introducing an element size factor  $\lambda_S$ , which is obtained by reproducing the tensile test in FE with different element size of  $L_{E,a}$ , where  $a \in [0, t]$  and t is the number of element sizes considered. The reference element size is  $L_{E,a} = L_{E,0}$ for a = 0. The final expression for  $\lambda_S$  is (Pavlović et al. 2013)

$$\lambda_S = \sqrt[3]{\frac{L_{E,0}}{L_{E,a}}}, \ a \in [1,t].$$
(23)

It is worth emphasising that  $\lambda_E$  is used to model damage evolution in the S355 steel. For the Grade 8.8 bolt, a linear damage evolution law is used for simplicity since it does not require the introduction of  $\lambda_E$ .

### 164 Calibration of $\lambda_E$

Tension loading of dog-bone specimens for the S355 steel were simulated to 165 calibrate  $\lambda_E$ . Four specimens with cylindrical cross-sectional area, and dimen-166 sions stipulated by the ASTM Standard E8/E8M-15a (2015), are discretised with 167 C3D8R (8 nodes linear continuum elements with reduced integration) elements of 168 a uniform size throughout its gauge section. The element sizes considered were: 169  $L_{E,0} = 0.89$ mm (186 elements across diameter),  $L_{E,1} = 1.00$ mm (140 elements), 170  $L_{E,2} = 1.25$ mm (96 elements) and  $L_{E,3} = 1.59$ mm (60 elements). Each element 171 size was chosen so that their aspect ratio is  $\approx 1$ . A displacement boundary condi-172 tion of 0.05 mm/s (corresponding to a nominal strain rate of  $0.001 \text{s}^{-1}$ ) is applied 173 to one end of the specimen, with the other end fully clamped, to simulate tensile 174 loading. The true stress-strain curve in Fig. 6 is used and damage initiation follows 175 Eq. 19. 176

<sup>177</sup> A flow chart summarising the procedure to calibrate  $\lambda_E$  is shown in Fig. 7. <sup>178</sup> An initial FE model discretised uniformly with C3D8R elements of size  $L_{E,0}$  is <sup>179</sup> first considered with a trial  $\lambda_E$  value.  $\lambda_S$  is, then, evaluated - note that  $\lambda_S = 1$ <sup>180</sup> (Eq. 23) for  $L_{E,a} = L_{E,0}$  - and Eqs. 20, 21 and 22 are used to obtain  $D_i$ ,  $\bar{u}_i^{\text{pl}}$ <sup>181</sup> and  $\bar{u}_f^{\text{pl}}$ , respectively. The predicted stress-strain curve is then compared to its experimental counterpart. If their differences  $Er(\varepsilon_r^{\text{nom}}) \leq 2\%$ , then the trial  $\lambda_E$ value is satisfactory and the same procedure repeated for  $L_{E,1}$ ,  $L_{E,2}$  and  $L_{E,3}$ . If, however,  $Er(\varepsilon_r^{\text{nom}}) > 2\%$  for any  $L_{E,a}$ , the initial value of  $\lambda_E$  is updated and the simulations repeated for all  $L_{E,a}$ . Pavlović et al. (2013) have shown that  $\lambda_E \in [2.5, 3.2]$  for C3D8R elements and the final value to use depends on the ductility of the material. Here, the calibrated value of  $\lambda_E = 2.5$  lies within the range stipulated by Pavlović et al. (2013).

The difference in the predicted nominal strain at rupture  $\varepsilon_r^{\text{nom}}$  with experiment is less than 2% for all  $L_{E,a}$ . A linear damage evolution law is assumed for the bolt, with an equivalent plastic displacement at fracture given by  $\bar{u}_f^{\text{pl}} = \varepsilon_f L_{\text{E}}$  (ABAQUS 2009).  $L_{\text{E}} = 2\text{mm}$  is used here so that there is a minimum of 12 to 16 nodes across the bolt diameter (Virdi 1999); and,  $\varepsilon_f = 0.13$  follows from Girão Coelho (2004). Figure 8 shows the damage initiation and evolution curves that were implemented into the FE models.

#### <sup>196</sup> Simulations of T-stub in tension and validation

Figure 9 shows a schematic of the tensile test simulated by FE. Recall that 197 WT1 and T1 comprises of two bolt-rows; hence, only a quarter of the T-stub was 198 simulated and  $u_z = 0$  must be specified for the x - y plane (Fig. 9a), unlike in 199 T15. The bottom web is fully clamped and a displacement boundary condition 200 of  $\dot{u}_y = 0.01 \text{mm/s}$  (Girão Coelho 2004) was applied to the top web. Table 3 201 lists the value of each geometric parameter shown in Figure 9b. Since T15 was 202 obtained from an IPE300 beam profile (Bursi and Jaspart 1997), there is no weld 203 to be modelled. By contrast, both WT1 (Girão Coelho 2004) and T1 (Ribeiro 204 et al. 2015) were constructed by welding two plates together. In our FE model, 205

both welds and flange are assumed to be made of the same material (Girão Coelho 206 2004; Girão Coelho et al. 2004; Ribeiro et al. 2013; Latour and Rizzano 2012). The 207 same is also assumed for T15 given that no information was provided by Ribeiro 208 et al. (2015) regarding the weld material. The entire FE model is discretised using 209 C3D8R elements (Latour and Rizzano 2012; Latour et al. 2014). The bolts are 210 modelled as a solid cylinder with an equivelent cross-sectional area of  $A_s$  (Bursi 211 and Jaspart 1997). Figure 10a shows the results of three element sizes (4, 2 and212 1.5mm) – the flange is meshed as in B4C5D4 of Fig. 10d (this will be justified 213 later) – that were used to mesh the bolt. Since a 1.5mm element predicted a 214 similar ultimate displacement  $\Delta_u$  – black circular dot in Fig. 10 – to its 2mm 215 counterpart, the latter was used to reduce computational cost. The mesh for the 216 bolt, used in subsequent parametric study, is shown in Fig. 9d. To determine the 217 mesh size for the flange, we partition the flange into four zones (A, B, C and D) 218 as shown in Fig. 10d. Zone A is discretised using C3D8R elements of different 219 sizes, while the rest of the flange was meshed as shown in B4C5D4 (Fig. 10d), 220 and their corresponding results are shown in Fig. 10b. After this, three mesh 221 densities were tested for zones B, C and D – see Fig. 10d – where the number of 222 elements along the x-axis is given after each letter. Figure 10a shows a negligible 223 difference between the predicted  $\Delta_u$  by B4C5D4 and B6C10D4. Hence, Region B 224 is discretised with four elements across the circle; while region C with four elements 225 between regions A & B and between regions B & D. Region D is discretised with 4 226 elements. The mesh for the flange, used in subsequent parametric study, is shown 227 in Fig. 9c. Surface-to-surface contact formulation with small sliding is prescribed 228 for all contact pairs – top and bottom flange, flanges and head/nut of the bolt, 229 bolt shank and hole – with a coefficient of Coulomb friction  $\mu = 0.25$  (Bursi and 230

<sup>231</sup> Jaspart 1997).

Figure 11 compares the predicted deformation history of the T-stubs to its 232 experimental counterpart. The resistance  $(F_u)$  of a T-stub is defined as the max-233 imum resultant reaction force acting at the bottom web and its corresponding 234 displacement is the ultimate displacement  $(\Delta_u)$  - they are shown in Fig. 11 by red 235 (experiment) and black (FEM) dots. The predicted force-displacement  $(F - \Delta)$ 236 curve closely matches that of the experiments where the percentage differences for 237  $F_u$  (WT1: 3.4%, T1: 3.1%, T15: 5.3%) and  $\Delta_u$  (WT1: 0.7%, T1: 5.1%, T15: 238 2.6%) are small. In addition, comparison can also be made for the bolt elongation 239  $\Delta_b$  in WT1 (note that this was not measured in T1 and T15) as shown in Fig. 11d. 240 The discrepancy in the final bolt elongation between experiment and FE arises be-241 cause the former was halted at  $\Delta_b = 0.4$  mm to prevent equipment damage (Girão 242 Coelho 2004). Notwithstanding, a good general agreement - differences of less 243 than 5.1% - is observed until  $\Delta_b = 0.4$  mm. The mode of failure is also successfully 244 predicted by the FE models; this is evident from the distributions of equivalent 245 plastic strain ( $\varepsilon^{\text{pl}}$ ) and damage variable ( $D_i$ ) shown in Fig. 11. Note that the con-246 tour plots for the flange and bolt correspond to the ultimate displacement. Both 247 WT1 and T15 failed in mode 2. A plastic hinge also develops at the weld-toe in the 248 FE model which is highlighted in Figs. 11a and c: here,  $\varepsilon^{\rm pl}$  exceeds the threshold of 249 0.05 through the thickness, as suggested by Ribeiro et al. (2015). In Figs. 11a and 250 c, the damage variable  $D_i = 1$  is reached in the bolt; consequently, elements are 251 removed from the mesh. Specimen T1 developed two plastic hinges; at both the 252 weld-toe and bolt hole. However, Fig. 11b also shows that  $D_i$  is close to unity in 253 the bolt. This suggests that the bolt had undergone significant plastic deformation 254 which agrees with observations by Bursi and Jaspart (1997) who suggested that 255

the actual failure mechanism is between mode 1 and 2. The location of fracture in the experiment was not indicated by Bursi and Jaspart (1997); however, the value of  $D_i$  in our FE model suggests that bolt fracture is imminent.

# 259 RESULTS

The FE model is now employed in a parametric study to investigate how the deformation capacity of a T-stub is affected by its geometric and material parameters; and, the results will also be used to critically assess the accuracy, and limitations, of two existing analytical models developed by Piluso et al. (2001) and Francavilla et al. (2016).

#### <sup>265</sup> Parametric study

### 266 Geometry and material properties

9b shows a schematic of the T-stub and the geometric dimensions Figure 267 considered. Note that the tensile response of a T-stub that develops beam yield 268 line pattern had been extensively studied, both experimentally and numerically, by 269 others; see, for example, Piluso et al. (2001), Girão Coelho (2004) and Ribeiro et al. 270 (2015). In addition, analytical models also exist that could accurately predict their 271 F -  $\Delta$  relationship (Piluso et al. 2001; Francavilla et al. 2016). By contrast, there 272 are relatively fewer studies on non-circular yield line patterns. For this reason, 273 all the T-stubs here were sized to develop this; hence, their width L must satisfy 274 L > 4m + 1.25n. Since B is fixed at 200mm in all the models, m and n are varied 275 to obtain three different  $\lambda ~(\triangleq n/m)$  values as follows: (1) A maximum value of 276  $\lambda_{\rm max} = 1.25$  determined by considering the minimum standard bolt spacing of 277  $p_{\min} = 98$ mm for a HEA200 beam (UNI 5397:1978 1978). This is taken as a 278 reference beam profile since it is characterised by a width which is identical to B279

(200mm). Note that if  $\lambda > \lambda_{\max}(= 1.25)$ , non-circular patterns will not develop according to McGuire and Winter (1978); (2) A minimum value of  $\lambda_{\min} = 0.9$ determined by considering the maximum allowable bolt spacing of  $p_{\max} = 14t_p$  as stipulated in EN 1993-1-8 (2005); (3) An intermediate value of  $\lambda_{\text{inter}} = 1.1$  - this was selected to lie between  $\lambda_{\max}$  and  $\lambda_{\min}$ . Three flange thicknesses  $t_p$  (8, 9 and 10mm) and four bolt diameters  $d_b$  (10, 12, 20 and 27mm) are considered. Table 4 tabulates the combinations of dimensions that were modelled.

Four different grades of structural steel (S235, S275, S355 and S450) are mod-287 elled for the flange material. Their respective true stress-strain curve is represented 288 using a piece-wise approximation similar to Piluso et al. (2001) in Fig. 3. Key val-289 ues of stresses ( $\sigma_y$  and  $\sigma_u$ ), strains ( $\varepsilon_y$ ,  $\varepsilon_h$ ,  $\varepsilon_u$  and  $\varepsilon_f$ ) and moduli (E,  $E_h$  and  $E_f$ ) 290 are tabulated in Table 5. For the S450 steel,  $\sigma_y$  and  $\sigma_u$  were obtained from EN 291 1993-1-8 (2005),  $E_f = \sigma_u$  (Piluso et al. 2001) and  $\varepsilon_f = 0.17$  (EN 10025-2 2004); 292 and,  $E, E_h, \varepsilon_y, \varepsilon_h$  and  $\varepsilon_u$  are assumed to be identical to those of S355. The dam-293 age initiation criterion for each steel is given by Eq. 19 and their corresponding 294 values tabulated in Table 6. Damage evolution is modelled as shown in Fig. 8 for 295 all the four grades of steel because the actual nominal stress-strain curves were not 296 provided by Girão Coelho (2004) for S235, S275 and S450. This is an acceptable 297 assumption given that all four steels are characterised by similar  $\varepsilon_u$  and  $\varepsilon_f$  (Table 298 5). The same Grade 8.8 bolt is used throughout and is modelled as previously 299 described. 300

301 Results and Discussions

Results of the parametric study are plotted in Fig. 12. Analytical predictions by Piluso et al. (2001) and Francavilla et al. (2016) are then compared to the

FE predictions of the ultimate displacement  $\Delta_u$  and failure mode for  $\lambda_{\text{max}} = 1.25$ 304 in Table 4. Since the analytical models are, hitherto, mostly applied only to 305  $\lambda \approx 1$ , comparison will be made here for  $\lambda_{\rm max} = 1.25$ . The deformation capacity 306 of a T-stub is charactersied by its non-dimensional ultimate displacement  $\delta \triangleq$ 307  $\Delta_u/t_p$ , and this is plotted against  $\Gamma \triangleq \kappa f_y/f_{ub} \cdot t_p^2/A_s$ . Notice that  $\Gamma$  is a product 308 of two dimensionless groups that were previously used to delineate the régime 309 boundaries. From Eq. 13, it is clear that  $\Gamma = h(\lambda)$ ; hence, for a constant  $\lambda$ , the 310 régime boundaries depend only on  $\Gamma$ . 311

Figure 12 shows a general reduction in the deformation capacity  $\delta$  with  $\Gamma$ 312 and/or when the mode switches from 1 to 3. It is hardly surprising that the 313 ductility of a T-stub in mode 1 is highest due to the collapse mechanism it develops. 314 And, since the collapse mechanism is affected by both geometric and material 315 parameters (EN 1993-1-8 2005), the ductility of T-stubs is highly dependent on 316 the dimensionless parameters  $t_p^2/A_s$  and  $kf_y/f_{ub}$ . Notice that the data points for 317 mode 1 are much more disperse, further confirming the sensitivity of ductility to 318 geometric parameters and material properties. By contrast, mode 3 deformation 319 is dictated by the deformation of the bolts and is, consequently, less sensitive to 320 geometric and material properties of the flange. 321

Table 4 tabulates the value of  $\delta$  for each T-stub. It can be seen that, for a constant  $\kappa f_y/f_{ub}$ ,  $\delta$  increases for T-stubs constructed with a weak flange and strong bolts. These T-stubs fail predominantly in either mode 1 or 2. By contrast, T-stubs with strong flanges and weak bolts deform primarily in mode 3, and they have low deformation capacity ( $\delta$ ) that is nearly constant for  $\Gamma > 1.5$ . Furthermore, Table 4 also shows that, for the same combination of  $t_p$  and  $A_s$ ,  $\delta$  reduces with increasing  $\kappa f_y/f_{ub}$  for all modes of failure. In addition, the effects of  $\kappa f_y/f_{ub}$  on  $\delta$  is

greatest for  $\lambda = \lambda_{\min}$  (mode 1: 5.3  $\leq \delta \leq 6$  and mode 2: 1.9  $\leq \delta \leq 4.5$ ) compared 329 to  $\lambda_{\text{inter}}$  (mode 1: 4.9  $\leq \delta \leq 5.5$  and mode 2: 1.9  $\leq \delta \leq 4.0$ ) and  $\lambda_{\text{max}}$  (mode 330 1:  $4.6 \leq \delta \leq 5.1$  and mode 2:  $1.4 \leq \delta \leq 3.6$ ). This is because when  $\lambda = \lambda_{\min}$ , 331 n is small compared to m which implies that the bolt spacing p is large. Girão 332 Coelho (2004) observed that a large p is responsible for a reduction of the T-stub 333 stiffness because the flange is not as rigidly constrained between the weld-toe and 334 bolt line compared to cases where  $\lambda = \lambda_{\max}$  (n >> m). Increasing stiffness of a 335 T-stub is accompanied by a consequential increase in its ductility – T-stubs with 336  $\lambda = \lambda_{\min}$  are more compliant – which leads to comparatively higher ductility of the 337 T-stub (Girão Coelho 2004). By contrast, the effects of both  $t_p^2/A_s$  and  $\kappa f_y/f_{ub}$ 338 are negligible on  $\delta$  for large  $\Gamma$  as the data points eventually flatten out. 339

Table 4 also compares the FE results for  $\lambda = \lambda_{\text{max}}$  to analytical predictions 340 by Piluso et al. (2001) and Francavilla et al. (2016). In general, both models 341 under-predict  $\delta$  in mode 1 because the deformation mechanism was assumed to be 342 two-dimensional. This simplification is only valid for a beam yield line pattern; 343 instead, a non-circular yield line pattern characterised by 3D effects develops in 344 the flange. The model by Piluso et al. (2001) over-predicts the displacement in 345 mode 2 because it neglects the compatibility condition between the elongation of 346 the bolt and the deformation of the flange; this was subsequently addressed by 347 Francavilla et al. (2016). Notwithstanding, discrepancies remain between FE and 348 analytical predictions in mode 2 because geometric non-linearities were neglected 349 in both analytical models. However, the difference between FE and analytical 350 predictions is small in mode 3, which suggests that both analytical models are, 351 in general, accurate if applied to T-stubs that undergo small displacements. The 352 good agreement in mode 3 is also, partly, because of the insensitivity of mode 3 353

<sup>354</sup> deformation to the yield line pattern that develops.

#### <sup>355</sup> Failure deformation maps - validation

The fidelity of the deformation maps is demonstrated for non-circular yield line 356 pattern in Fig. 13 by plotting the data from the parametric study; and for the beam 357 yield line pattern with experimental data from Girão Coelho (2004), Piluso et al. 358 (2001), Bursi and Jaspart (1997) and Ribeiro et al. (2015). Plastic (dotted lines) 359 and ultimate (solid lines) régime boundaries are plotted for  $\lambda_{\min}$  (Fig. 13a),  $\lambda_{inter}$ 360 (Fig. 13b) and  $\lambda_{\rm max}$  (Fig. 13c). Note that the boundaries corresponding to mode 361  $2\rightarrow 3$  for  $\lambda_{\text{max}}$  lie below the ones for  $\lambda_{\text{min}}$ , similarly in Figs. 4 and 5. This is because 362 T-stubs with a smaller  $\lambda$  value tend to be more ductile; consequently, they are more 363 likely to fail either in mode 1 or 2. Only a limited combinations of  $t_p^2/A_s$  and  $f_y/f_{ub}$ 364 causes mode 3 failure. The plastic failure mode predicted by the map is, in general, 365 conservative since T-stubs that fail in mode 1 (or 2) were predicted to fail in mode 366 2 (or 3). This is unsurprising given that the régime boundaries in the plastic failure 367 map were constructed within the constitutive framework of limit analysis (for the 368 flange material). By contrast, the ultimate régime boundaries were constructed 369 by assuming a linear piece-wise approximation of the stress-strain curve (Fig. 3) 370 which gives a better prediction of the failure mode. The data points indicated 371 by blue arrows in Fig. 13 – they were identified by their row number in Table 4 372 and the flange material – are outliers due to the approximate nature of the piece-373 wise idealisation of the flange material. Apart from the outliers, the failure maps 374 predict well the mode of deformation predicted by the parametric study. 375

The maps also demonstrate how failure mode is influenced by  $t_p^2/A_s$  and  $\kappa f_y/f_{ub}$ . Increasing  $t_p^2/A_s$  leads to a shift in the mode of failure from 1 to 3 which is evident from the columns of data – each column corresponds to a constant  $\kappa f_y/f_{ub}$  – in Fig. 13. Similarly, increasing  $\kappa f_y/f_{ub}$  at a constant  $t_p^2/A_s$  value also leads to a shift towards a less ductile mode  $(1 \rightarrow 2 \text{ or } 2 \rightarrow 3)$ . It is worth noting that since  $f_{ub}$ is identical in the parametric study, increasing  $\kappa f_y/f_{ub}$  corresponds to a stronger flange. If the flange has an increased resistance to deformation, then the bolt is more likely to fail even for lower  $t_p$  and higher  $A_s$  values - see, for example, data points 19 (S450), 23 (S355) and 23 (S450) in Fig. 13.

To examine the effects of varying  $A_s$  whilst keeping  $t_p$  constant, consider the 385 T-stub configurations 1 to 4 listed in Table 4. Figure 13(c) shows that a T-stub 386 with a greater  $A_s$  tends to fail in mode 1 – as exemplified by rows of data points 387 labelled 3 and 4 – while a smaller  $A_s$  leads to failure in mode 2 or 3 (exemplified 388 by rows of data points 1 and 2). The plastic failure map – their régime boundaries 389 are plotted as dotted lines – suggests that one would need to increase the diameter 390 of the bolt in order for the T-stub to deform in mode 1. This is in contrast to what 391 the ultimate failure map would suggest. Hence, using the ultimate map prevents 392 the over-sizing of bolts which is a common, yet expensive, strategy adopted by 393 structural steel designers. If instead one focuses on configurations 1, 5 and 9 394 – they are characterised by a constant  $A_s$  – it is evident from Fig. 13(c) that 395 increasing  $t_p$  leads to a less ductile failure mode; see, for example, the rows of data 396 points labelled 5 and 9. 397

Table 7 compares the predicted deformation using the maps to existing experimental test data. It is clear that the plastic régime boundaries are excessively conservative as they tend to predict a mode 2 deformation for T-stubs failing in mode 1. The ultimate boundaries are evidently more accurate and are capable of subdividing the geometric  $(t_p^2/A_s)$  and material  $(\kappa f_y/f_{ub})$  parameters space into 403 correct modes of failure that are consistent with the experimental data.

Finally, the maps are only as good as the theory (specifically, the constitutive idealisation of the flange material) used to construct them. But they are useful in spite of their inexactness for both designing and interpreting experiments, and in selecting and understanding the behaviour of T-stubs for engineering applications. And, by identifying the places where data or theory are poor, they can be systematically improved.

## 410 CONCLUSIONS

Failure deformation maps were constructed for the plastic and ultimate failure 411 of a T-stub with a single bolt-row in tension. The maps allow to avoid iterative 412 pre-design calculations by condensing a large body of information within the 2D 413 parameters space  $t_p^2/A_s - \kappa f_y/f_{ub}$ . It was found that the failure mode is sensitive 414 to the non-dimensional parameters  $t_p^2/A_s$  and  $\kappa f_y/f_{ub}$ . The maps show that a 415 ductile failure mode (mode 1) is induced by either decreasing  $t_p$  or increasing  $A_s$ . 416 The fidelity of the maps is demonstrated through existing experimental data and 417 through a FE parametric investigation. It was shown that the analytical models 418 by Piluso et al. (2001) and Francavilla et al. (2016) under-predict the ultimate 419 displacement in mode 1 arising from the assumption of a beam yield line pattern. 420 Both models are also shown to be accurate if applied to T-stubs undergoing small 421 displacements were geometric non-linearities are negligible. 422

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FIG. 1. Schematic of failure modes adapted from Ribeiro et al. (2015). Q is prying force and B is tensile force in the bolt.



FIG. 2. Possible yield line patterns in a T-stub: (a) non-circular; (b) circular; and (c) beam. --- shows hinge line and L is the width (adapted from Girão Coelho (2004)).



FIG. 3. Idealised piece-wise approximation of the true stress-strain curve of the flange material (Piluso et al. 2001).



FIG. 4. Plastic failure map. Black and red isolines are régime boundaries at mode  $1\rightarrow 2$  and mode  $2\rightarrow 3$  transition, respectively. Solid lines for non-circular and dotted lines for beam yield line patterns.



FIG. 5. Ultimate failure map. Black and red isolines are régime boundaries at mode  $1\rightarrow 2$  and mode  $2\rightarrow 3$  transition, respectively. Solid lines for non-circular and dotted lines for beam yield line patterns.



FIG. 6. Material properties for S355 steel and Grade 8.8 bolt.



FIG. 7. Flow-chart on the procedure to calibrate  $\lambda_E$ .



FIG. 8. (a) Damage initiation criterion and (b) damage evolution law.



FIG. 9. (a) FE model of the T-stubs and (b) its corresponding geometric parameters. The FE meshes for bolt and flange are shown in (c) and (d).  $r_w = 0.8a_w\sqrt{2}$  for WT1 and T15;  $r_w = 0.8r$  for T1.



FIG. 10. Results of mesh sensitivity study for WT1 (Girão Coelho 2004), where black dot indicates ultimate displacement  $\Delta_u$ : (a) effects of element size in the bolt shaft; (b) effects of element size at the weld toe; (c) predicted **F** -  $\Delta$  curves by modelling the flange as shown in (d).



FIG. 11. Comparison between FE (——) and experiments (- - -). Arrow indicates deleted elements in the bolt and dotted circle indicates where plastic hinge develops.



FIG. 12. Results of parametric study: (a)  $\lambda_{\min} = 0.9$ , (b)  $\lambda_{inter} = 1.11$  and (c)  $\lambda_{\max} = 1.25$ . Vertical lines indicate boundaries between ultimate failure modes.



FIG. 13. Predicted failure mode by FE for (a)  $\lambda_{min}=0.9$ , (b)  $\lambda_{inter}=1.11$  and (c)  $\lambda_{max}=1.25$ . Mode 1 ( $\bullet$ ), Mode 2 ( $\bullet$ ) and Mode 3 ( $\bullet$ ). Grey numbers identify the geometry (Table 4) and flange materials are indicated.

Mode transition	β
$1 \rightarrow 2$	$\frac{2\lambda}{1+2\lambda}$
$2 \rightarrow 3$	2

TABLE 1: Values of  $\beta$  delineating the transition between failure modes given by Piluso et al. (2001).

		$h(\lambda)$			
	Mode transition	Non-circular	Beam	$\kappa$	
	1 . 9	$3.57\lambda$	$1.44\lambda$	1	
Plastic limit state	$1 \rightarrow 2$	$\overline{(1+2\lambda)(4+1.25\lambda)}$	$\overline{(1+2\lambda)}$	1	
	$2 \rightarrow 3$	$2 \rightarrow 3 \qquad \qquad \frac{3.57}{1}$			
		$4 + 1.25\lambda$			
	1 . 9	$6\lambda$	$2.4\lambda$	$\mathbf{F}_{\alpha}$ 10	
Ultimate limit state	$1 \rightarrow 2$	$\overline{(4+1.25\lambda)(1+2\lambda)}$	$\overline{1+2\lambda}$	Eq. 10	
	$2 \rightarrow 3$	6	2.4	Eq. 10	
	_ /0	$(4+1.25\lambda)$		-1, 10	

TABLE 2: Expressions for  $h(\lambda)$  in Eq. 13.

Model	m	n	$t_p$	$d_b$	$l_1$	$l_2$	L	В	p	$a_w/r$	$t_w$	$r_w$
WT1	34.34	30	10	12	20	25	45	150.08	50	5	10	5.7
T1	29.45	30	10.7	12	20	20	40	150	90	15	7.1	12
T15	42.1	30	15	20	52.5	52.5	105	170	110	7	10	7.9

TABLE 3: Geometric dimensions corresponding to Fig. 9b. All dimensions are in mm except  $a_W/r$ .

		G	eomet	ry			Prec	licted $\delta$	
Row	m	n	$\lambda$	$t_p$	$d_b$	S235	S275	S355	S450
1	40	50	1.25	8	10	2.3(5.0,2.0)	1.8(3.8, 1.6)	1.2(1.4,0.7)	0.7(0.9, 0.5)
2	40	50	1.25	8	12	3.6(5.0, 2.3)	3.0(3.8, 2.1)	2.2(3.6, 2.6)	1.5(2.6, 1.57)
3	40	50	1.25	8	20	5.1(5.2,5.1)	4.9(4.0, 3.8)	4.7(3.9,3.7)	4.6(3.34, 3.53)
4	40	50	1.25	8	27	4.6(5.2,5.0)	4.1(4.0, 3.6)	3.8(3.9, 3.5)	3.5(3.34, 2.65)
5	40	50	1.25	9	10	1.5(1.9,1.4)	1.1(1.2,0.4)	0.6(0.3, 0.3)	0.3(0.4, 0.18)
6	40	50	1.25	9	12	2.7(4.0, 1.4)	2.1(2.5,1.4)	1.5(1.5,1.4)	1.0(0.92, 0.35)
7	40	50	1.25	9	20	4.9(4.1, 4.1)	4.2(3.2, 4.6)	4.9(3.1,3.7)	3.5(2.6,2.1)
8	40	50	1.25	9	27	4.4(4.1,3.9)	4.0(3.2,2.3)	3.9(3.1,2.7)	3.7(2.7, 2.16)
9	40	50	1.25	10	10	0.8(0.3,0.2)	0.4(1.1,0.2)	0.3(0.5, 0.3)	0.2(0.3, 0.3)
10	40	50	1.25	10	12	1.7(3.2,1.4)	1.3(1.3,1.1)	0.9(0.7, 0.3)	0.4(0.1, 0.2)
11	40	50	1.25	10	20	3.8(3.4, 3.9)	3.3(2.6, 4.2)	3.0(3.1,2.5)	2.3(2.5,3.0)
12	40	50	1.25	10	27	4.1(3.4,3.2)	3.9(2.6, 2.4)	3.8(2.6,2.3)	3.5(2.2, 3.5)
13	44	46	1.1	8	10	2.6	2.01	1.4	0.9
14	44	46	1.1	8	12	4.0	3.4	2.7	2.1
15	44	46	1.1	8	20	5.5	5.3	5.1	4.9
16	44	46	1.1	8	27	5.1	4.6	4.0	3.8
17	44	46	1.1	9	10	2.5	2.3	0.5	0.3
18	44	46	1.1	9	12	3.0	1.6	1.3	0.3
19	44	46	1.1	9	20	5.6	5.4	5.3	5.0
20	44	46	1.1	9	27	4.8	4.8	3.9	3.6
21	44	46	1.1	10	10	1.2	0.9	0.5	0.3
22	44	46	1.1	10	12	1.8	2.3	0.8	0.7
23	44	46	1.1	10	20	4.5	3.9	1.9	3.5
24	44	46	1.1	10	27	4.7	4.5	4.4	4.2
25	47	43	0.9	8	10	3.0	2.3	1.6	1.1
26	47	43	0.9	8	12	4.5	3.8	3.1	1.9
27	47	43	0.9	8	20	6.0	5.9	5.7	5.4
28	47	43	0.9	8	27	5.7	5.1	4.0	4.4
29	47	43	0.9	9	10	2.0	2.6	0.3	0.6
30	47	43	0.9	9	12	3.2	1.8	1.3	0.4
31	47	43	0.9	9	20	5.9	5.5	4.9	3.5
32	47	43	0.9	9	27	5.0	4.6	4.3	4.2
33	47	43	0.9	10	10	1.5	0.6	0.4	1.1
34	47	43	0.9	10	12	2.0	1.6	1.0	0.9
35	47	43	0.9	10	20	5.2	4.5	4.0	3.5
36	47	43	0.9	10	27	4.7	4.3	4.3	3.9

TABLE 4: Dimensions (mm) of T-stubs - see Fig. 9b - considered in the parametric study.  $l_1 = l_2 = L/2 = 125$ mm; B = 200mm;  $t_w = 6.5$ mm;  $a_w = 5$ mm and  $r_w = 7.5$ mm in all the models. XX (YY, ZZ) refers to FEM (Piluso et al. (2001), Francavilla et al. (2016)).

Steel	$\sigma_y$ (MPa)	$\sigma_u$ (MPa)	$\varepsilon_y$	$\varepsilon_h$	$\varepsilon_u$	$\varepsilon_{f}$	E (MPa)	$\begin{array}{c} E_h \\ \text{(MPa)} \end{array}$	$\begin{array}{c} E_f \\ (\text{MPa}) \end{array}$	$\kappa$
S235	235	360	0.001	0.014	0.036	0.25	210000	5500	360	2.587
S275	275	430	0.001	0.015	0.047	0.22	210000	4800	430	2.560
S355	355	510	0.002	0.017	0.053	0.2	210000	4250	510	2.305
S450	440	550	0.002	0.017	0.053	0.17	210000	4250	550	2.305

TABLE 5: Key stresses, strains, moduli and  $\kappa$  corresponding to Fig. 3.

S	235	Sž	275	S	355	S450		
θ	$\bar{\varepsilon}_0^{\mathrm{pl}}$	$\theta$	$\bar{\varepsilon}_0^{\mathrm{pl}}$	θ	$\bar{\varepsilon}_0^{\mathrm{pl}}$	θ	$\bar{\varepsilon}_0^{\mathrm{pl}}$	
0.1	0.183	0.1	0.158	0.1	0.144	0.1	0.128	
0.13	0.176	0.13	0.156	0.13	0.143	0.13	0.130	
0.33	0.130	0.33	0.112	0.33	0.102	0.33	0.090	
0.67	0.078	0.67	0.067	0.67	0.061	0.67	0.054	
1	0.048	1	0.041	1	0.037	1	0.033	
1.33	0.029	1.33	0.025	1.33	0.022	1.33	0.020	
1.67	0.114	1.67	0.015	1.67	0.014	1.67	0.012	
2	0.010	2	0.009	2	0.008	2	0.007	

TABLE 6: Damage initiation criteria.

	Ge	eome	try	N	lateria	al	Failure Mode		
Model	$t_p$	$d_b$	λ	$\kappa$	$f_y$	$f_{ub}$	Exp.	Plas.	Ult.
1	14.4	20	1.04	3.12	291	800	2	2	2
2	14.6	20	1.04	3.33	265	800	2	2	2
3	13	20	0.52	3.49	273	800	2	2	2
4	12.3	24	1.19	3.68	300	800	1	2	1
6	16.4	24	0.99	3.70	280	800	2	2	2
9	12.5	27	0.99	3.58	301	800	1	1	1
12	12.2	20	0.93	2.95	347	800	1	2	1
WT1	10	12	0.9	2.31	355	800	1	2	2
$WT7_M20$	10	20	0.9	3.33	355	800	1	2	1
$WT7_M16$	10	16	0.9	3.33	355	800	1	2	2
$WT57_M16$	10	16	0.9	1.74	690	800	2	2	2
$WT57_M12$	10	12	0.9	1.74	690	800	2	2	2
WT7_M12	10	12	0.9	3.33	355	800	2	2	2
T1			1.02				1	2	2
T15			0.7				2	2	2

TABLE 7: Comparison of predicted failure mode (by the deformation maps) against experiment data from (a) Piluso et al. (2001), (b) Girão Coelho (2004), (c) Bursi and Jaspart (1997) and (d) Ribeiro et al. (2015).